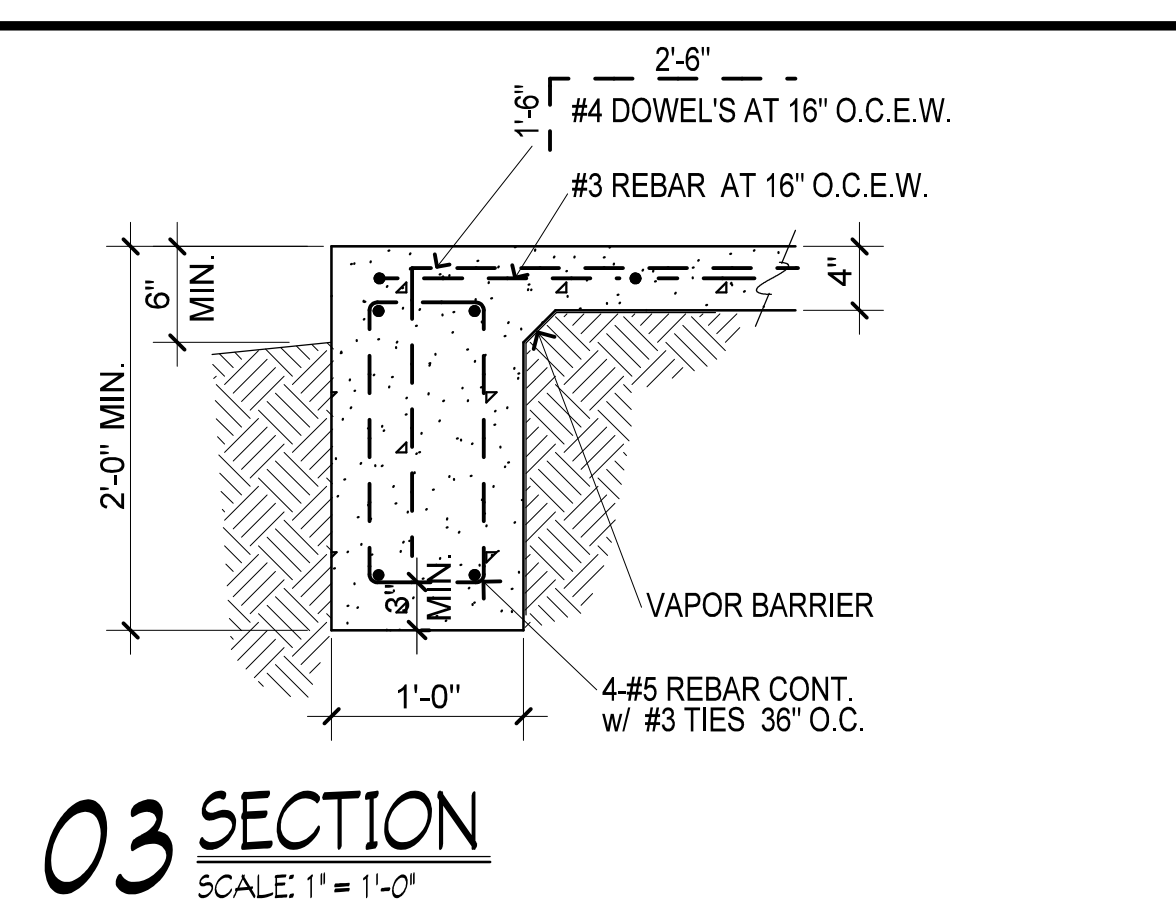
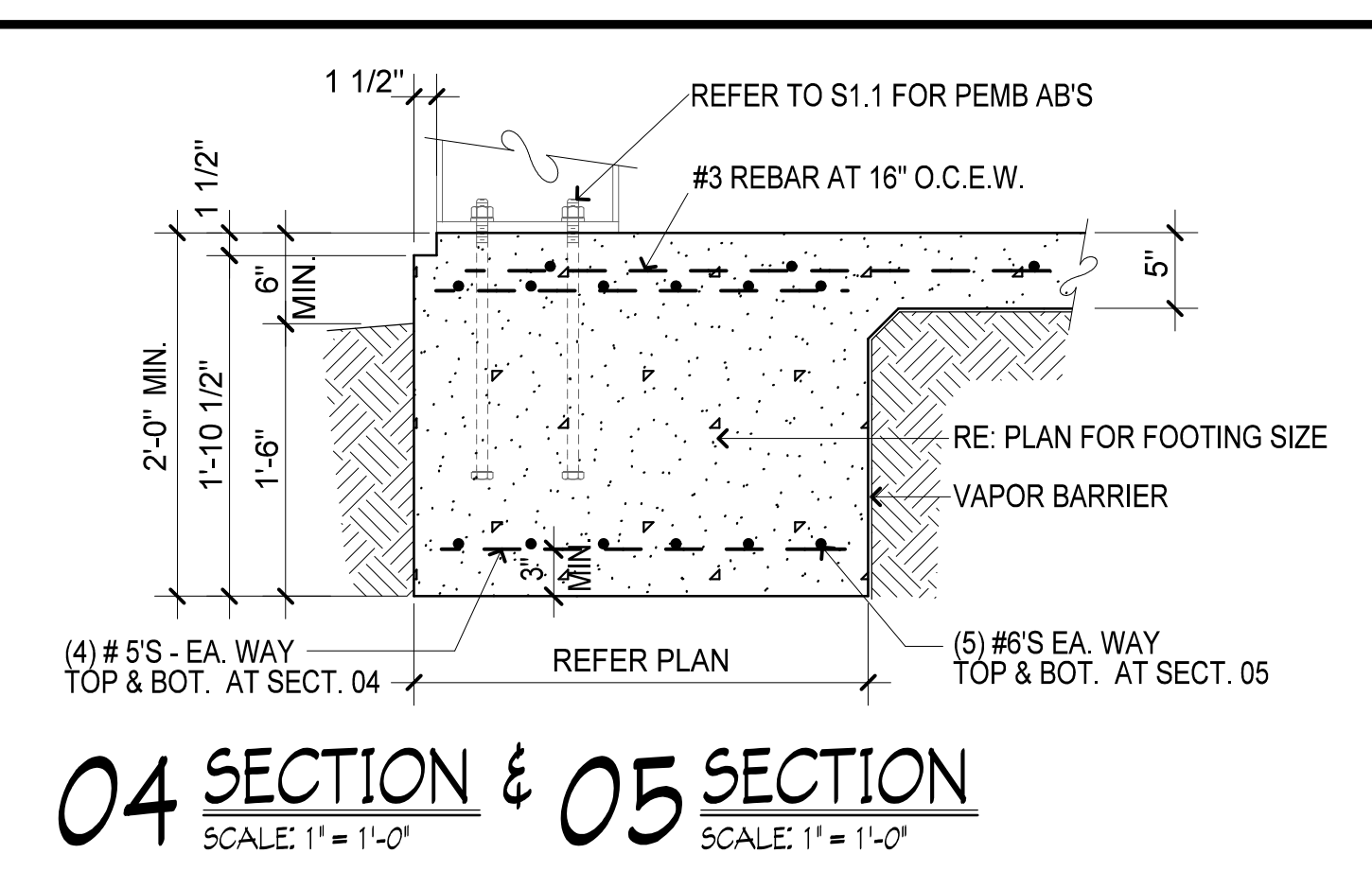


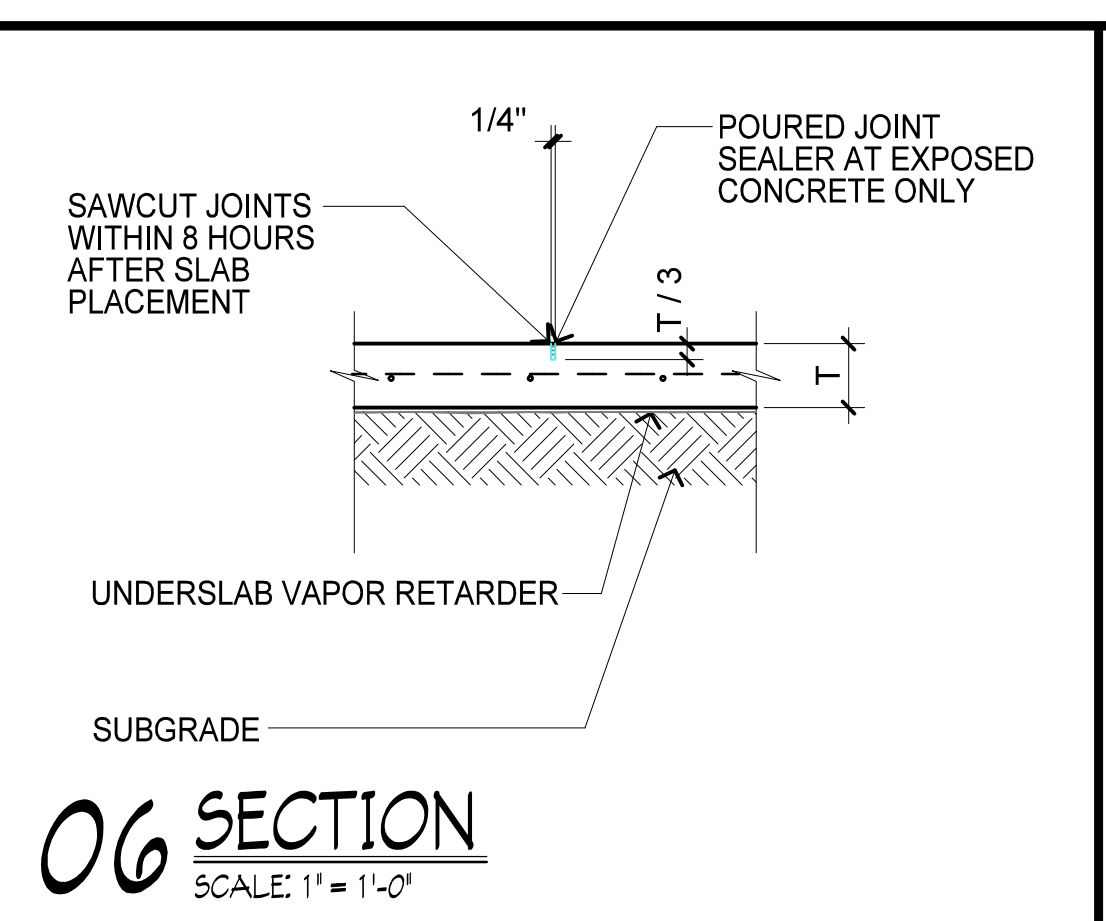
**02 SECTION**  
SCALE: 1" = 1'-0"



**03 SECTION**  
SCALE: 1" = 1'-0"



**04 SECTION & 05 SECTION**  
SCALE: 1" = 1'-0"



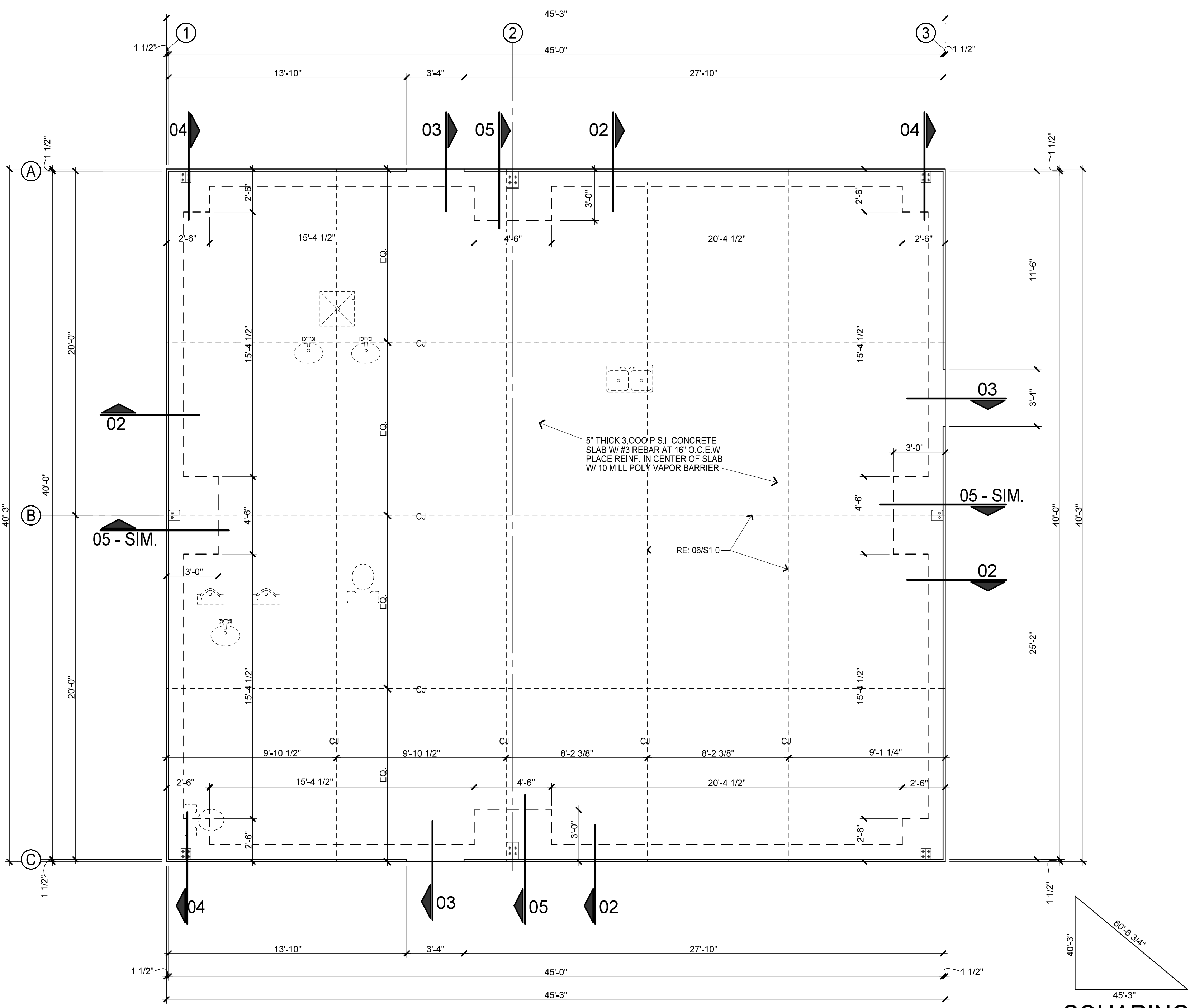
**06 SECTION**  
SCALE: 1" = 1'-0"

**PLAN NOTES**

- TYPICAL SLAB NOTE:**  
5" CONCRETE SLAB OVER 4" SAND CUSHION. REINF. WITH #3 @ 16" O.C.E.W., PLACE REINF. IN CENTER OF SLAB. PROVIDE JOINT SEALER UNDER SLAB.
- FURNISH AND INSTALL 10 MIL ASTM E 1745 CLASS A VAPOR RETARDER UNDER ALL INTERIOR SLABS ON GRADE. VAPOR RETARDER SHALL BE STEGO WRAP OR APPROVED ALTERNATE. PERMEANCE AS TESTED AFTER MANDATORY CONDITIONING LESS THAN 0.02 PERMS.
- VAPOR RETARDER SHEETS SHALL BE INSTALLED OVER LEVEL, COMPACTED 4" SAND CUSHION
- WITH 6" JOINT LAPS. JOINT LAPS SHALL BE CONTINUOUSLY SEALED WITH TAPE PROVIDED BY MANUFACTURER. SEAL CAREFULLY AROUND ALL PIPES, CONDUITS, ETC. WHICH PENETRATE THE SLAB. SEAL ALL TEAR AND PUNCTURES PRIOR TO PLACING CONCRETE.
- BUILDING SUBGRADE PREPARATION:**  
ALL SUBGRADE PREPARATION AND SELECT FILL OPERATIONS SHALL BE IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION.
- TYPICAL SLAB CONTROL JOINT**  
PROVIDE SAW-CUT JOINTS PER DETAIL 1/51.1. CUT JOINTS WITHIN 8 HOURS AFTER CONCRETE PLACEMENT. MAX 15' X 15' SPACING. REFER TO FOUNDATION PLAN FOR JOINT LOCATIONS.

**STRUCTURAL NOTES**

- CODES:**  
1. BUILDING CODE: 2012 INTERNATIONAL BUILDING CODE  
2. STRUCTURAL CONCRETE: BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, AMERICAN CONCRETE INSTITUTE, ACI 318
- DESIGN LOADS:**  
1. DESIGN LOADS:  
ULTIMATE WIND SPEED — 115 MPH  
EXPOSURE — "C"  
RISK CATEGORY: II
- SITE DRAINAGE**  
1. WATER SHALL NOT BE ALLOWED TO COLLECT NEAR THE FOUNDATION PAD OF THE BUILDING EITHER DURING OR AFTER CONSTRUCTION. WATER SHALL NOT BE ALLOWED TO POND IN THE UNDERCUT OR EXCAVATED AREAS UNDER THE BUILDING. PROPER DRAINAGE SHALL BE COORDINATED WITH THE GEOTECHNICAL ENGINEER DURING SUBGRADE PREPARATION.
- FOUNDATIONS**  
1. FOUNDATION DESIGN IS BASED UPON THE FOLLOWING DESIGN PARAMETERS:  
ALLOWABLE BEARING PRESSURE:  
SHALLOW FOOTINGS — 1500 PSF AT A DEPTH OF 2.0 FEET BELOW FINISH SLAB SUBGRADE.  
2. REFER TO GEOTECHNICAL INVESTIGATION FOR GREGG COUNTY PRECINCT 3, LONGVIEW, TEXAS, DATED SEPTEMBER 19, 2016, AS PREPARED BY HOUSE ENGINEERING, LONGVIEW, TEXAS, PROJECT JOB # 335-173.  
3. FOOTINGS AND GRADE BEAMS SHALL BE DESIGNED TO BEAR IN PROPERLY COMPACTED SELECT FILL EXTENDING A MINIMUM DEPTH OF 2 FEET BELOW THE EXISTING SUBGRADE OR ADJACENT EXTERIOR GRADE (WHICHEVER IS DEEPER), AND PLACED SELECT FILL SHALL EXTEND MINIMUM OF FIVE (5) FEET BEYOND THE BUILDING AREA. MINIMUM FOUNDATION WIDTHS FOR COLUMN AND STRIP FOOTINGS SHALL BE 24 INCHES AND 12 INCHES, RESPECTIVELY. FOOTINGS SHALL BE PROPORTIONED FOR ALLOWABLE GROSS BEARING PRESSURES OF 1,500 PSF FOR INDIVIDUAL (ISOLATED) FOOTINGS AND 2,000 PSF FOR CONTINUOUS (STRIP) SHALLOW FOOTINGS.  
4. ALL SITE AND SUBGRADE PREPARATION AND STRUCTURAL FILL SHALL BE IN ACCORDANCE WITH THE ABOVE REFERENCED GEOTECHNICAL REPORT.  
5. WATER SHALL NOT BE ALLOWED TO COLLECT NEAR FOUNDATION PAD OF THE BUILDING EITHER DURING OR AFTER CONSTRUCTION. WATER SHALL NOT BE ALLOWED TO POND IN THE UNDERCUT OR EXCAVATED AREAS UNDER THE BUILDING. PROPER DRAINAGE SHALL BE COORDINATED WITH THE GEOTECHNICAL ENGINEER DURING SUBGRADE PREPARATION.  
6. FOOTING EXCAVATIONS SHALL BE DRY AND CLEAN WHEN CONCRETE IS PLACED.
- BUILDING SUBGRADE PREPARATION**  
1. AT BUILDING, REMOVE VEGETATION, TOP SOIL, ROOTS, DEBRIS AND ANY SOFT SOILS BELOW THE OVEREXCAVATION LINE. THESE SOILS SHALL BE PROOF ROLLED, DISPOSED OR STOCKPILED FOR LATER USE IN LANDSCAPING.  
2. PROOF ROLL VERIFICATION: AFTER STRIPPING AND UNDERCUTTING, AS REQUIRED BY THE GRADING PLAN, AND OVEREXCAVATION AS REQUIRED BY THE GEOTECHNICAL REPORT, THE AREA SHALL BE PROOF ROLLED AS DIRECTED BY THE GEOTECHNICAL ENGINEER. ALL PROOF ROLLING AND UNDERCUTTING ACTIVITIES SHALL BE WITNESSED BY THE GEOTECHNICAL TECHNICIANS AND SHALL BE PERFORMED DURING A PERIOD OF DRY WEATHER.  
3. WEAK AREAS WHICH YIELD UNDER PROOF ROLLING, OR ANY AREAS WITH A TENDENCY TO PUMP, SHALL BE REWORKED TO PROVIDE STABLE SUPPORT AS DIRECTED BY THE GEOTECHNICAL ENGINEER.  
4. SCARIFICATION: AFTER STRIPPING, EXCAVATING AND PROOF ROLLING, BUT PRIOR TO PLACING FILL, THE EXPOSED SOILS SHALL BE SCARIFIED AND PROCESSED TO A MOISTURE CONTENT BETWEEN ONE PERCENTAGE POINT BELOW OPTIMUM (-1%) TO THREE PERCENTAGE POINTS ABOVE (+3%) THE STANDARD PROCTOR OPTIMUM. THE SUBGRADE SOILS SHALL BE RECOMPACTED TO A MINIMUM DENSITY OF 98% OF THE MAXIMUM DENSITY DEFINED BY ASTM D 698 (STANDARD PROCTOR), FOR A DEPTH OF EIGHT INCHES MINIMUM BELOW THE SURFACE.  
5. SELECT FILL: SELECT FILL MEETING THE FOLLOWING CHARACTERISTICS SHALL BE PLACED IN MAXIMUM LIFTS OF EIGHT INCHES OF LOOSE MATERIAL, AND SHALL BE COMPACTED AT A MOISTURE CONTENT ADJUSTED TO: PERCENT OF OPTIMUM, AND AT DENSITY OF AT LEAST 98% OF THE MAXIMUM DENSITY AS DETERMINED BY THE STANDARD PROCTOR (ASTM D 698) TEST. IF WATER MUST BE ADDED, IT SHALL BE UNIFORMLY APPLIED AND THOROUGHLY MIXED INTO THE SOIL BY DISKING OR SCARIFYING.  
• FREE OF ORGANIC OR OTHER DELETERIOUS MATERIAL.  
• HOMOGENEOUS MIXTURE.  
• MAXIMUM PARTICLE SIZE OF THREE (3) INCHES.  
• LIQUID LIMIT LESS THAN 35.  
• PLASTICITY INDEX BETWEEN FIVE (5) AND 18.  
• CONSIST OF LOW PLASTICITY SANDY CLAYS (CL) OR CLAYEY SANDS (SC) AS DEFINED BY THE UNIFIED SOIL CLASSIFICATION SYSTEM.  
6. ALL EARTHWORK OPERATIONS MUST BE UNDER THE SUPERVISION OF THE GEOTECHNICAL ENGINEER.  
7. CONDUCT IN-PLACE DENSITY TESTS FOR FILL BENEATH FOUNDATIONS AT THE RATE OF AT LEAST ONE TEST PER 2500 SQUARE FEET OF SURFACE AREA PER LIFT, OR A MINIMUM OF FOUR (4) TESTS PER LIFT FOR EACH TESTED AREA.
- CONCRETE REINFORCEMENT**  
1. ALL CONCRETE REINFORCEMENT SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A615 GRADE 60 AND UTILIZE CONTINUOUS STOCK LENGTH OF 40'-0" WHENEVER POSSIBLE. ALL REINFORCING BARS SHALL BE LAPPED 48 BAR DIAMETERS (2'-0" MINIMUM) AT SPLICES UNLESS NOTED OTHERWISE ON PLANS OR DETAILS. DEVIATIONS IN SPLICE LOCATIONS AND LENGTHS ARE NOT ALLOWED UNLESS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.  
2. CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH ACI 315 AND ACI 318.  
3. ALL HOOKS AND BENDS IN REINFORCING BARS SHALL CONFORM TO ACI DETAILING STANDARDS UNLESS SHOWN OTHERWISE.  
4. PROVIDE REINFORCING BARS IN ACCORDANCE WITH THE BAR BENDING DIAGRAM F BAR TYPES AS SPECIFIED. IN UNSCHEDULED BEAMS, SLABS, COLUMNS AND WALLS DETAIL REINFORCING AS FOLLOWS:  
a. LAP TOP REINFORCING BARS AT MIDSPAN.  
b. LAP BOTTOM REINFORCING BARS AT THE SUPPORTS.  
c. LAP VERTICAL BARS IN COLUMNS AND WALL ONLY AT FLOOR LINES, UNLESS NOTED OTHERWISE.  
d. LAP REINFORCING BARS 40 BAR DIAMETERS MINIMUM, UNLESS NOTED OTHERWISE.  
e. PROVIDE STANDARD HOOKS IN TOP BARS AT CANTILEVER AND DISCONTINUOUS ENDS OF BEAMS, WALLS AND SLAB.  
f. PROVIDE CORNER BARS FOR ALL HORIZONTAL BARS AT THE INSIDE AND OUTSIDE FACES OF INTERSECTING BEAMS OR WALLS. CORNER BARS ARE NOT REQUIRED IF HORIZONTAL BARS ARE HOOKED.  
5. WELDING OF REINFORCING STEEL WILL NOT BE PERMITTED.  
6. HEAT SHALL NOT BE USED IN THE FABRICATION OR INSTALLATION OF REINFORCEMENT.  
7. HOOK ALL TOP BARS 1'-0".
- PRE-ENGINEERED METAL BUILDINGS**  
1. METAL BUILDINGS SHALL BE DESIGNED AND DETAILED BY THE METAL BUILDING VENDOR.  
2. THE METAL BUILDING VENDOR SHALL SUBMIT COMPLETE DESIGN CALCULATIONS, SHOP DRAWINGS, DETAILS AND SPECIFICATIONS TO THE ARCHITECT FOR APPROVAL BEFORE FABRICATION AND ERECTION OF METAL BUILDING.  
3. METAL BUILDINGS SHALL BE DESIGNED TO CONFORM TO THE FOLLOWING LOADS OR MBMA SPECIFICATIONS, WHICHEVER IS MORE CRITICAL:  
DEAD LOAD — ACTUAL WEIGHT  
COLLATERAL LOAD — 3 PSF  
ROOF LIVE LOAD — 20 PSF (REDUCIBLE)  
WIND LOAD — REFER TO DESIGN LOADS  
4. CALCULATED DEFLECTION LIMITS - COMPONENTS TO WITHSTAND DESIGN LOADS WITH DEFLECTIONS NO GREATER THAN THE FOLLOWING:  
BUILDING BARE FRAME (DRIFT): HORIZONTAL DEFLECTION OF H/180  
ROOF PURLINS AND GIRDERS: VERTICAL DEFLECTION OF L/240 - TL  
VERTICAL DEFLECTION OF L/300 - LL  
METAL WALL GIRTS: HORIZONTAL DEFLECTION OF L/180 (METAL PANEL FINISH)  
5. ALL STRUCTURAL STEEL SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH LATEST MBMA SPECIFICATION UNLESS NOTED OTHERWISE.  
6. METAL BUILDING VENDOR SHALL DESIGN THE SUPERSTRUCTURE ASSUMING PINNED BASE CONNECTIONS TO THE FOUNDATION AND SHALL INCLUDE ANCHOR BOLT DESIGN AND DETAILS WITH THE SUPERSTRUCTURE PACKAGE.  
7. PRE-ENGINEERED ROOF STRUCTURE SHALL SUPPORT VENEER WALLS LATERSALLY AT TOP OF WALL WITH HORIZONTAL GIRTS/SPANDREL BEAMS PROVIDED BY METAL BUILDING VENDOR.  
8. PRE-ENGINEERED ROOF STRUCTURE SHALL PROVIDE LATERAL SUPPORT FOR INTERIOR METAL STUD WALLS.  
9. LETTER OF DESIGN CERTIFICATION SIGNED AND SEALED BY A QUALIFIED PROFESSIONAL ENGINEER. INCLUDE THE FOLLOWING:  
a. NAME AND LOCATION OF PROJECT  
b. NAME OF MANUFACTURER  
c. NAME OF CONTRACTOR  
d. BUILDING DIMENSIONS INCLUDING WIDTH, LENGTH, HEIGHT AND ROOF SLOPE.  
e. INDICATE COMPLIANCE WITH AISC STANDARDS FOR HOT-ROLLED STEEL AND SISI STANDARDS FOR COLD-ROLLED STEEL, INCLUDING EDITION DATES OF EACH STANDARD.  
f. GOVERNING BUILDING CODE AND YEAR OF EDITION.  
g. DESIGN: INCLUDE DEAD LOAD, ROOF LIVE LOAD, COLLATERAL LOADS, ROOF SNOW LOAD, DEFLECTION CRITERIA, WIND LOADS/SPEEDS AND EXPOSURE, SEISMIC DESIGN CATEGORY OR EFFECTIVE PEAK VELOCITY-RELATED ACCELERATION/PEAK ACCELERATION, AND AUXILIARY LOADS (CRANES).  
h. LOAD COMBINATIONS: INDICATE THAT LOADS WERE APPLIED ACTING SIMULTANEOUSLY WITH CONCENTRATED LOADS, ACCORDING TO GOVERNING BUILDING CODE.  
i. BUILDING-USE CATEGORY: INDICATE CATEGORY OF BUILDING USE AND ITS EFFECT ON LOAD IMPORTANCE FACTORS.



**01 FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"



PROJECT: New Training Facility  
OWNER: Gregg County Precinct 3  
Commissioner Gary Boyd  
ADDRESS: 6174 F.M. 2206, Longview, Texas 75604

DATE	REVISIONS

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**FOUNDATION PLAN**

**S1.0**

BY: HWK  
DATE: 29 November 2016

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