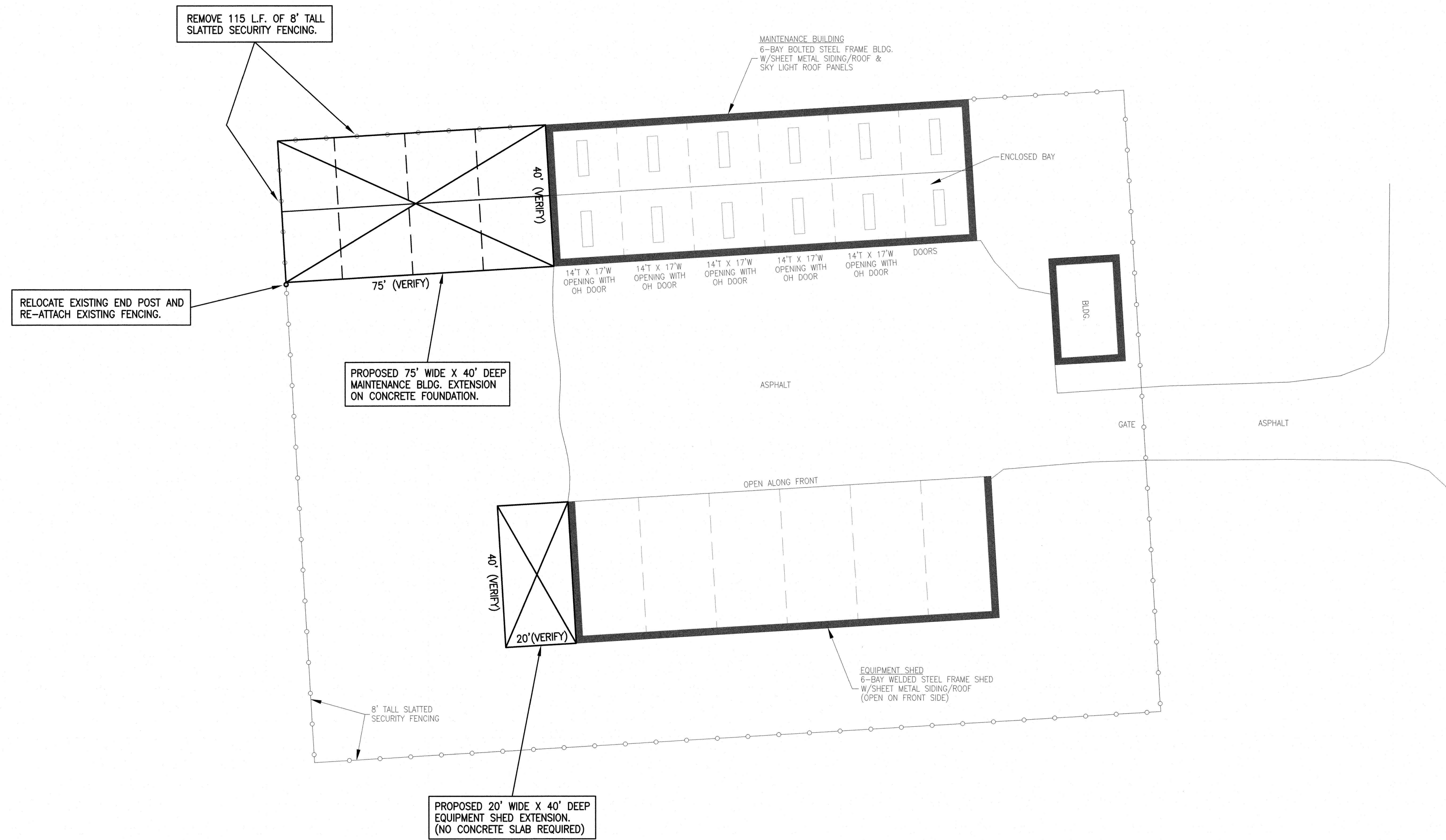


EAST TEXAS REGIONAL AIRPORT

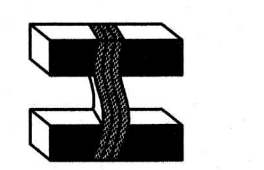


EAST TEXAS REGIONAL AIRPORT

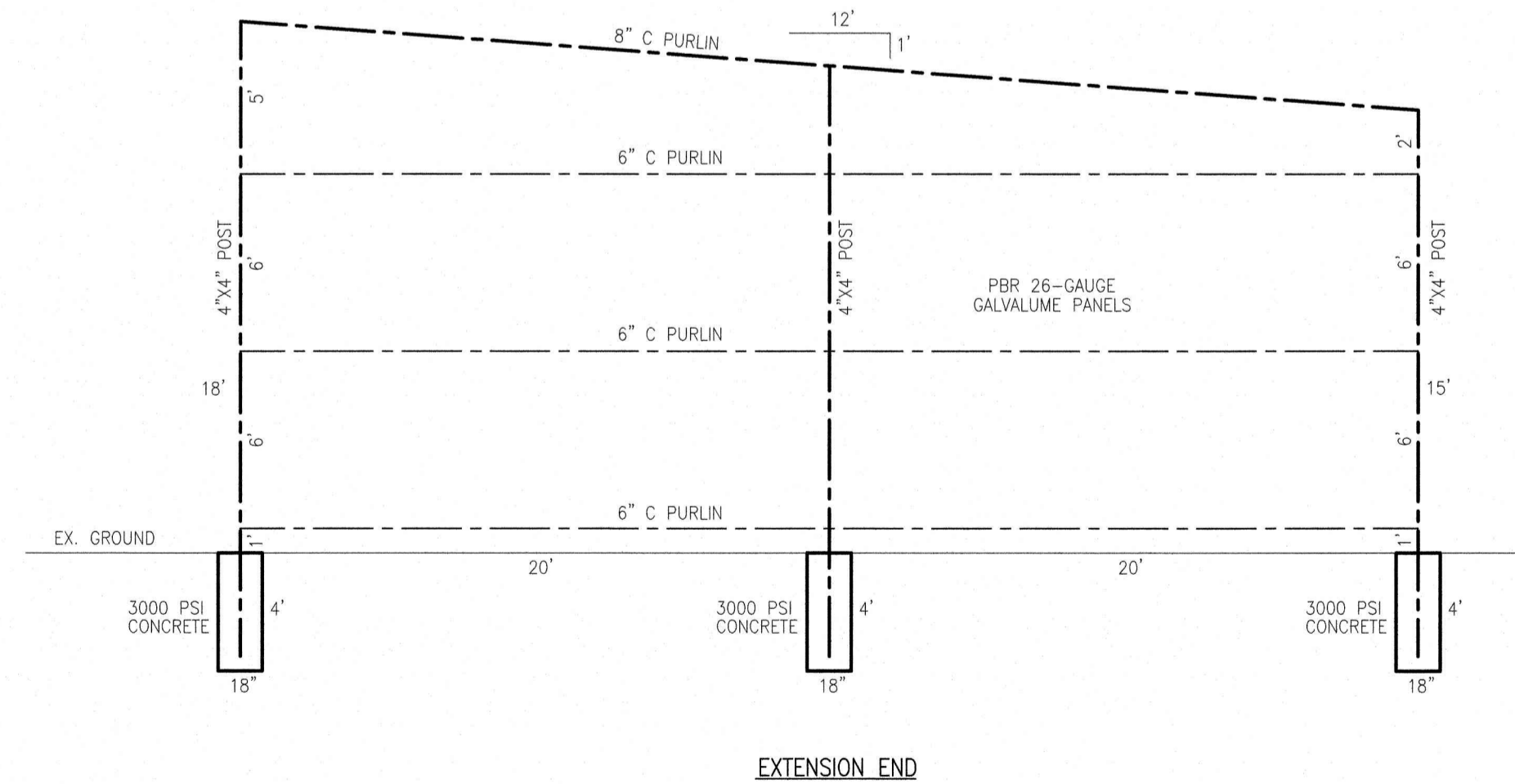
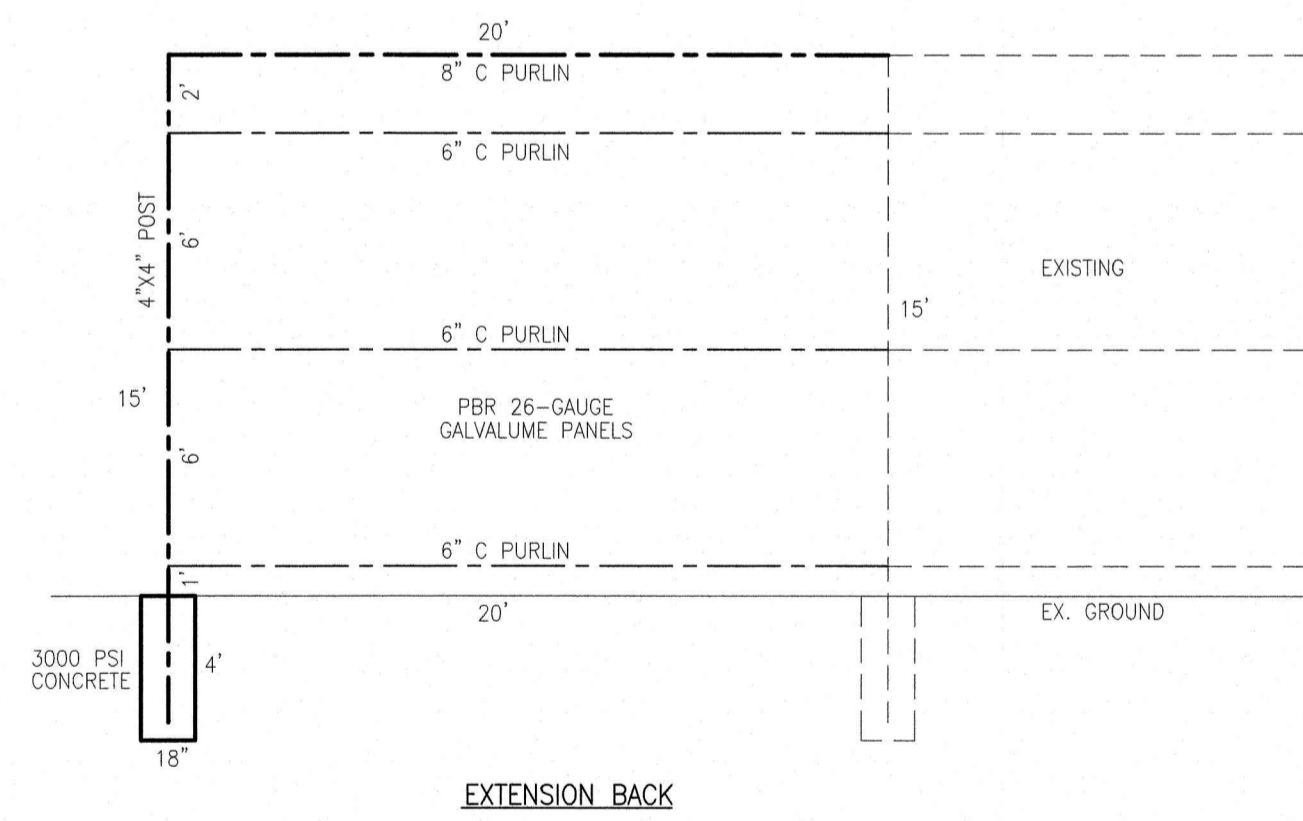
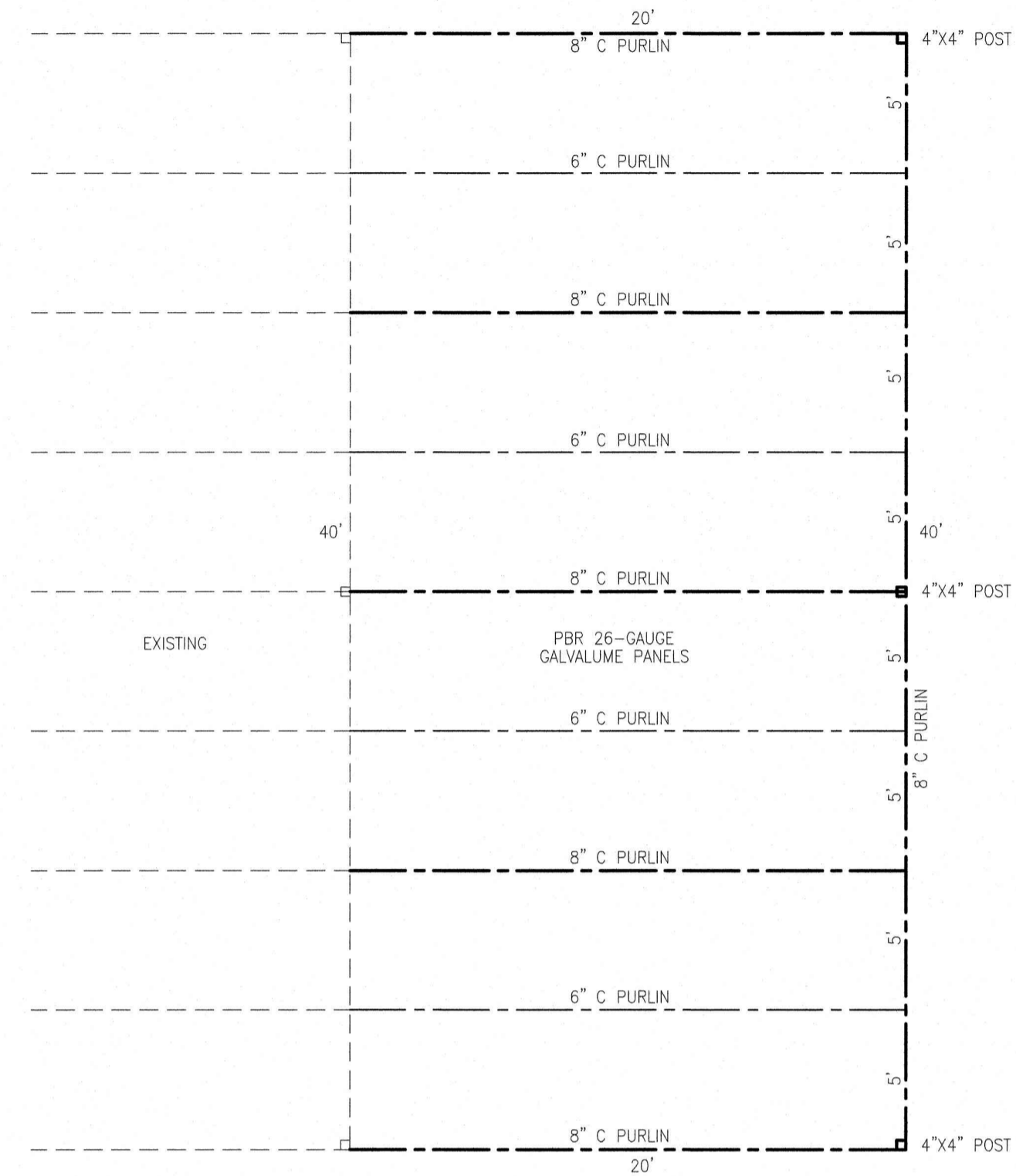
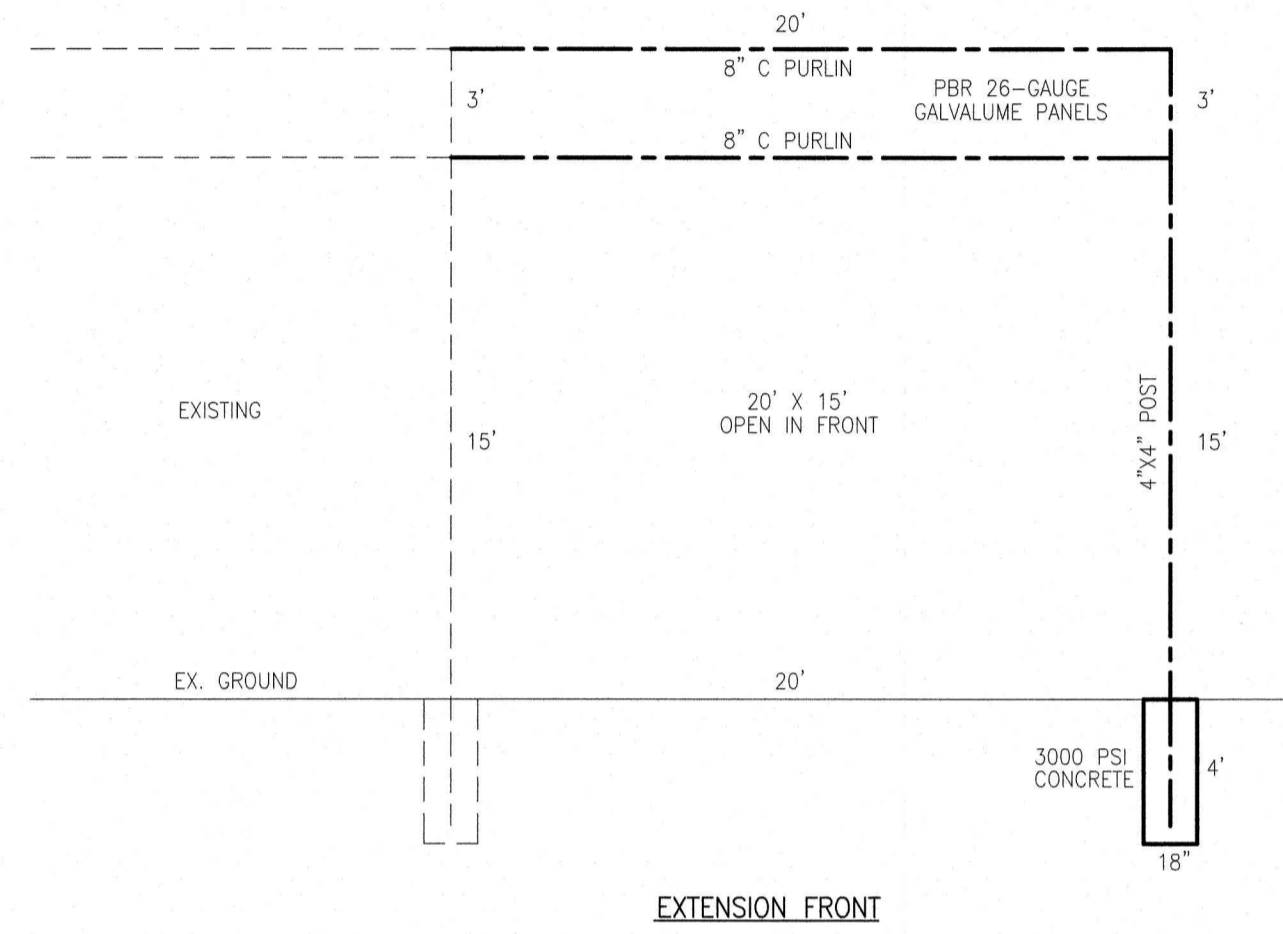
SITE PLAN

EAST TEXAS REGIONAL AIRPORT
GREGG COUNTY, TEXAS
MAINTENANCE BUILDING EXTENSION
AND EQUIPMENT SHED EXTENSION

HAYES ENGINEERING, INC.
Texas Registered Engineering Firm F-1465
2126 Alpine St. Longview, TX 75601-3401
Tel.: (903) 758-2010 • Fax: (903) 758-2099



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DATE : OCT. 2021
SCALE : NONE
JOB NO. : GREGG-21-01



GENERAL NOTES:

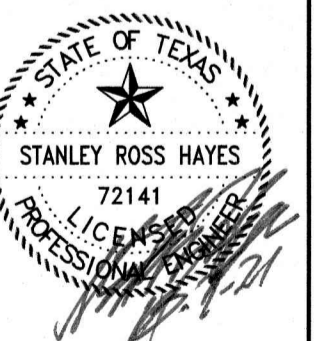
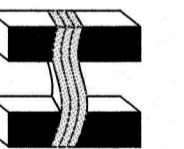
1. CONTRACTOR TO COORDINATE ALL CONSTRUCTION ACTIVITIES WITH EXTRA STAFF.
2. NEW SHED EXTENSION TO BE BUILT ON EXISTING GROUND.
3. ALL NEW MATERIALS TO MATCH MATERIALS IN EXISTING SHED.
4. CONSTRUCTION METHOD TO BE WELDED STEEL FRAME.
5. INSTALL TRIM AND FLASHING TO MATCH EXISTING SHED.
6. CONTRACTOR SHALL REMOVE EXISTING PBR PANELS ON END OF EXISTING EQUIPMENT SHED. THESE MATERIALS MAY BE RE-USED ON NEW END SECTION, PROVIDED THEY ARE NOT DAMAGED DURING REMOVAL AND STORAGE.
7. CONTRACTOR TO VERIFY ALL DIMENSIONS AND MATCH EXISTING EQUIPMENT BAY DIMENSIONS.

**EQUIPMENT SHED
EXTENSION**

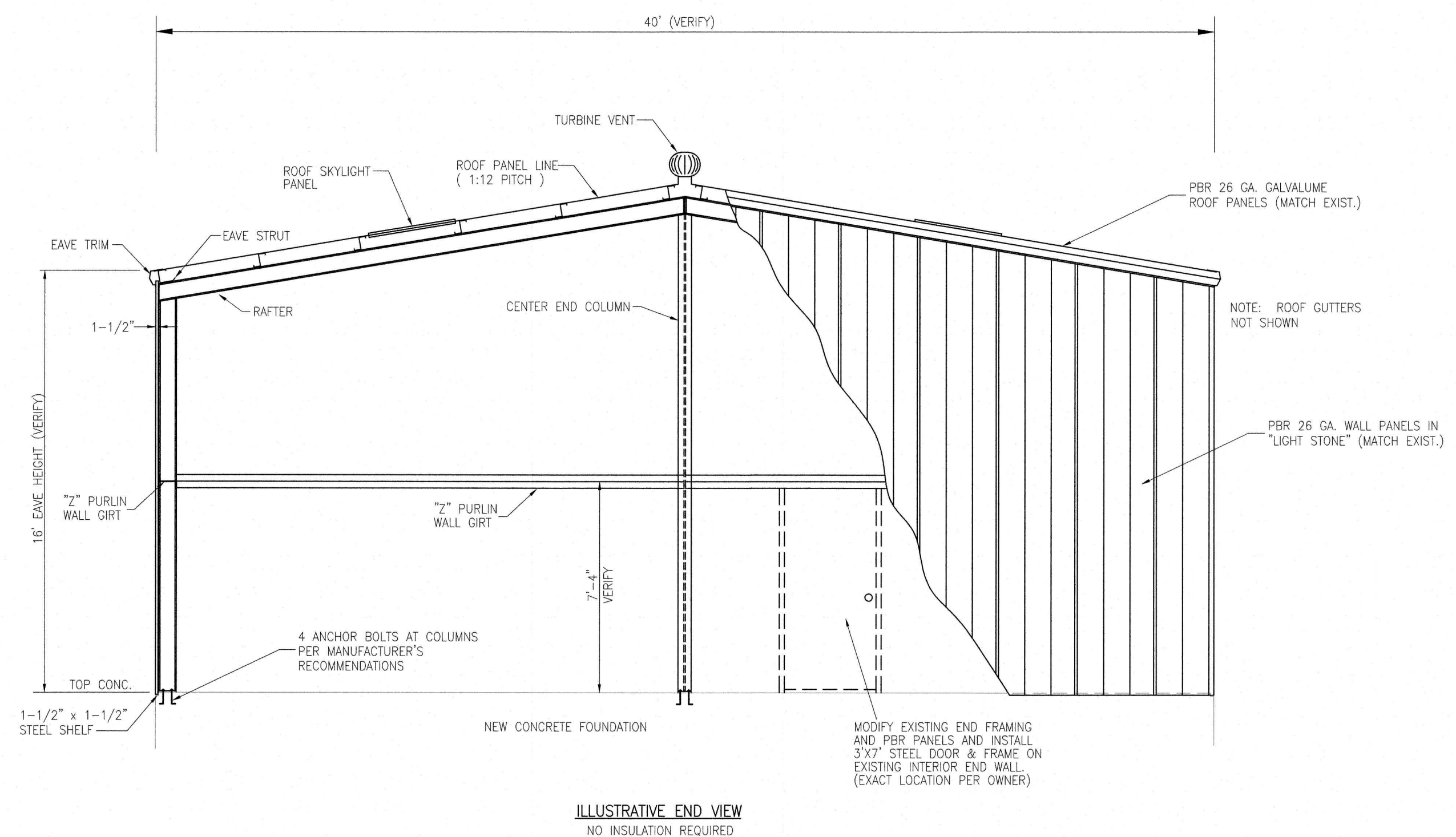
EAST TEXAS REGIONAL AIRPORT
GREGG COUNTY, TEXAS
**MAINTENANCE BUILDING EXTENSION
AND EQUIPMENT SHED EXTENSION**

HAYES ENGINEERING, INC.

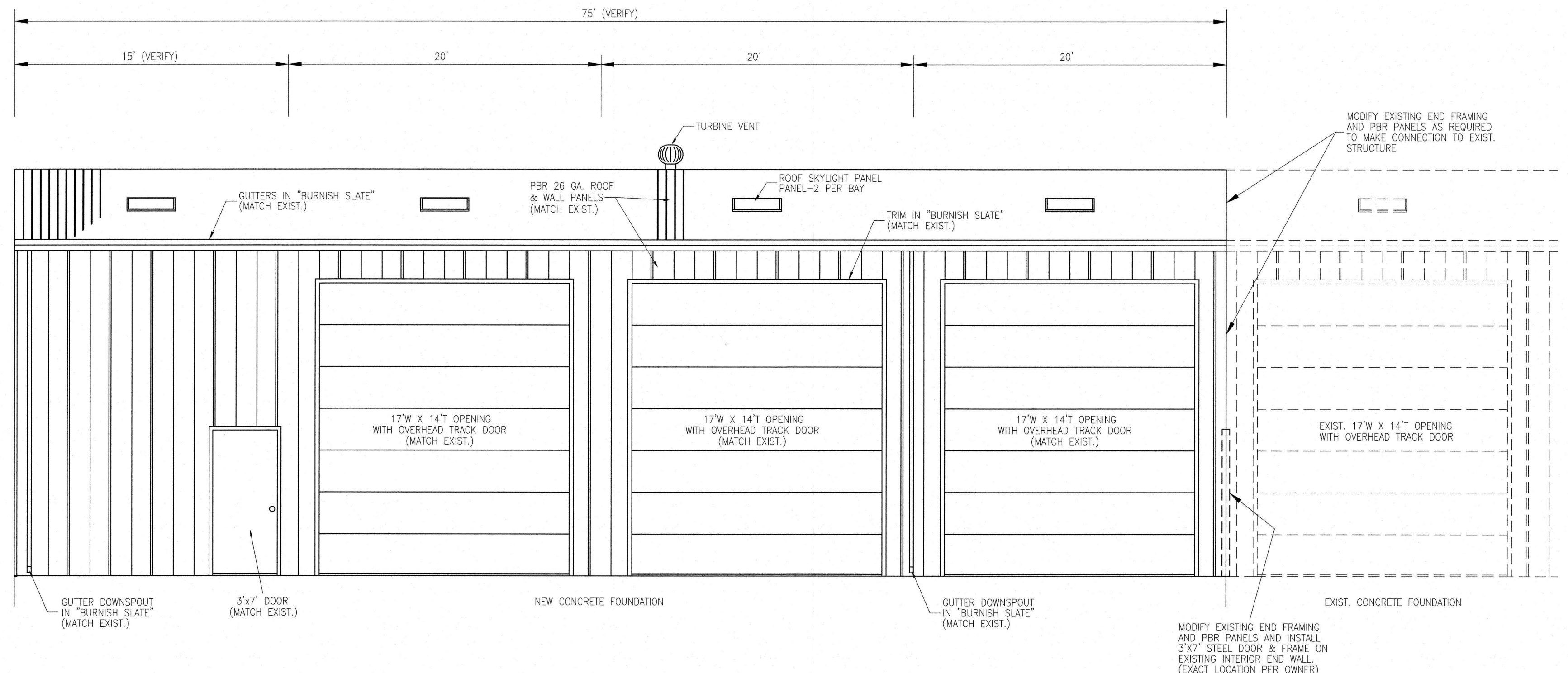
Texas Registered Engineering Firm E-1465
2126 Alpine St. Longview, TX 75601-3401
Tel: (903) 756-2010 • Fax: (903) 756-2099



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ILLUSTRATIVE END VIEW
NO INSULATION REQUIRED

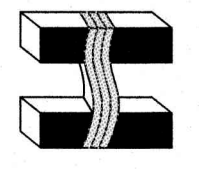


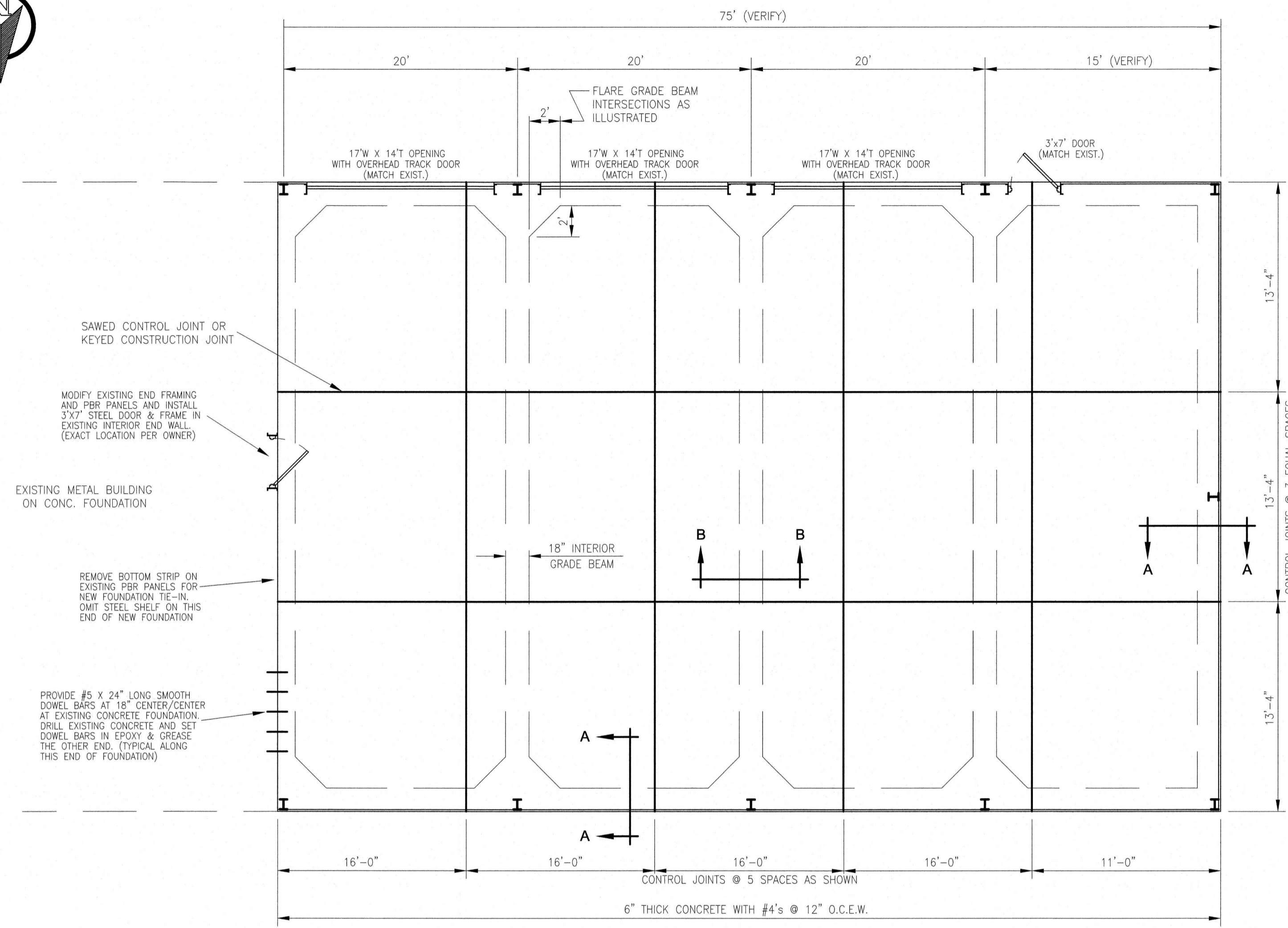
ILLUSTRATIVE FRONT VIEW
NO INSULATION REQUIRED

METAL BUILDING GENERAL NOTES

Design Requirements

- 1.6.1 Metal Building Systems, including structural framing, columns, eave struts and girts must be designed by a registered Professional Engineering licensed by the State of Texas. All drawings for the metal building systems must be signed and sealed by such Engineer.
- 1.6.2 All structural steel sections and welded plate members shall be designed in accordance with the applicable sections, relating to design requirements and allowable stresses, of the latest edition of the "AISC Specification for the Design, Fabrication and Erection of Structural Steel Buildings."
- 1.6.3 All light-gage, cold-formed, structural members and coverings shall be designed in accordance with the applicable sections, relating to design requirements and allowable stresses, of the latest edition of the "AISI Specification for the Design of Cold-Formed Sheet Structural Members."
- 1.6.4 Design Loads: The basic design loads shall be as follows:
 - 1.6.4.1 Dead load - self weight of the supported elements and components.
 - 1.6.4.2 Live load - 20 pounds per square foot f horizontal area.
 - 1.6.4.3 Wind load - 90 mph.
- 1.6.5 Application of loads: The design loads shall be applied to the structure as follows:
 - 1.6.5.1 Live Load and Dead Load - applied to entire horizontal projection.
 - 1.6.5.2 Wind load - the wind load shall be considered to act either inward or outward in the design of the wall and roof components, purlins, eave struts and girts applied to the frame as pressure on the windward wall, uplift on the roof and suction on the leeward wall, as recommended in the MBMA Design Practices Manual.
 - 1.6.5.3 Uplift - applied upwardly and normally to the roof.
- 1.6.6 Design load combinations: The design loads shall be applied to the structure in the following combinations:
 - 1.6.6.1 Dead Load + Live Load
 - 1.6.6.2 Dead + Wind Load
 - 1.6.6.3 Dead Load + Uplift
- 1.6.7 Building anchorage and foundation: The building anchor bolts and related anchorage shall be designed to resist the column reactions resulting from the specified loads. The floor slab and perimeter grade beams can provide for some continuity at the column bases, if required to limit member sizes or lateral drift of the rigid frames. The size, placement and design of the column bases shall be coordinated by the Structural Engineer prior to installation of drilled piers.
- 1.6.8 Exterior roof to withstand imposed loads with maximum allowable deflection of span: L/240.
- 1.6.9 Rigid frames and endwall framing to withstand imposed loads with maximum allowable deflection of H/500.
- 1.6.10 Size and fabricate roof systems free of distortion or defects detrimental to appearance or performance.
- 1.11 Regulatory Requirements
 - 1.11.1 In order for the Owner to be able to acquire windstorm and hail insurance through the Texas Catastrophe Property Insurance Association (T.C.P.I.A.) the State and applicable City require that the metal building structure in this contract must be inspected by a Professional Engineer registered in the State of Texas. Such engineer must certify that the building complies with the requirements of the Texas State Board of Insurance "Windstorm Resistant Construction Guide" (effective June 1, 1989 or most recent version).
 - 1.11.2 Toward this end, the metal building manufacturer shall provide (and shall include in his bid the cost of the services of a Professional Engineer registered in Texas to make inspections during the construction of the building sufficient for him to sign, at the completion of construction, the State Board of Insurance "Building Certificate, Form WPI-2" certifying that the building was constructed in accordance with the requirements of reference "Windstorm Resistant Construction Guide."
 - 1.11.3 The contract must certify and guarantee in writing prior to the start of construction, that such inspections will be made (at no additional cost to the Owner) and that at the completion of construction the required "Building Certificate" will be signed and provided as per State and City requirements.
 - 1.11.4 Certification of the building foundation will not be a part of the metal building manufacturer's responsibility.
 - 1.11.5 Cooperate with regulator agency or authority and provide data as requested



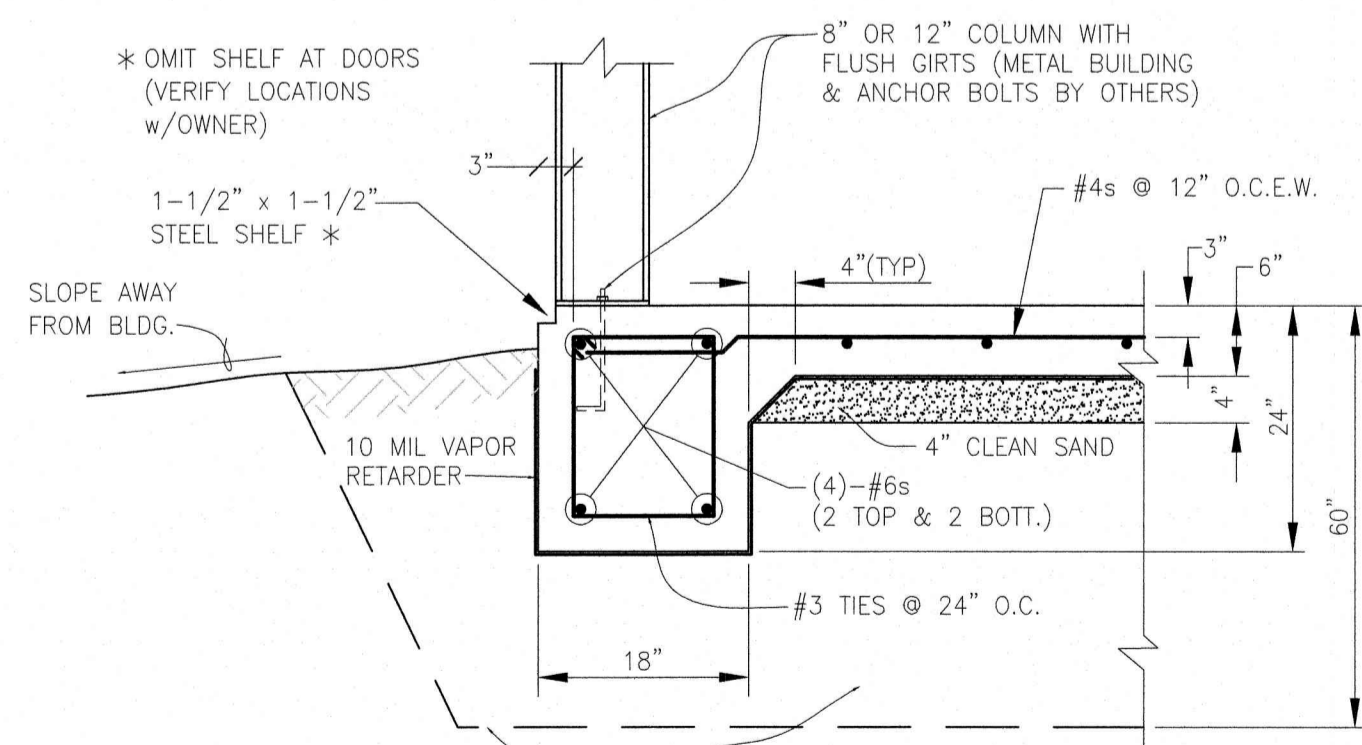


FOUNDATION PLAN

N.T.S.

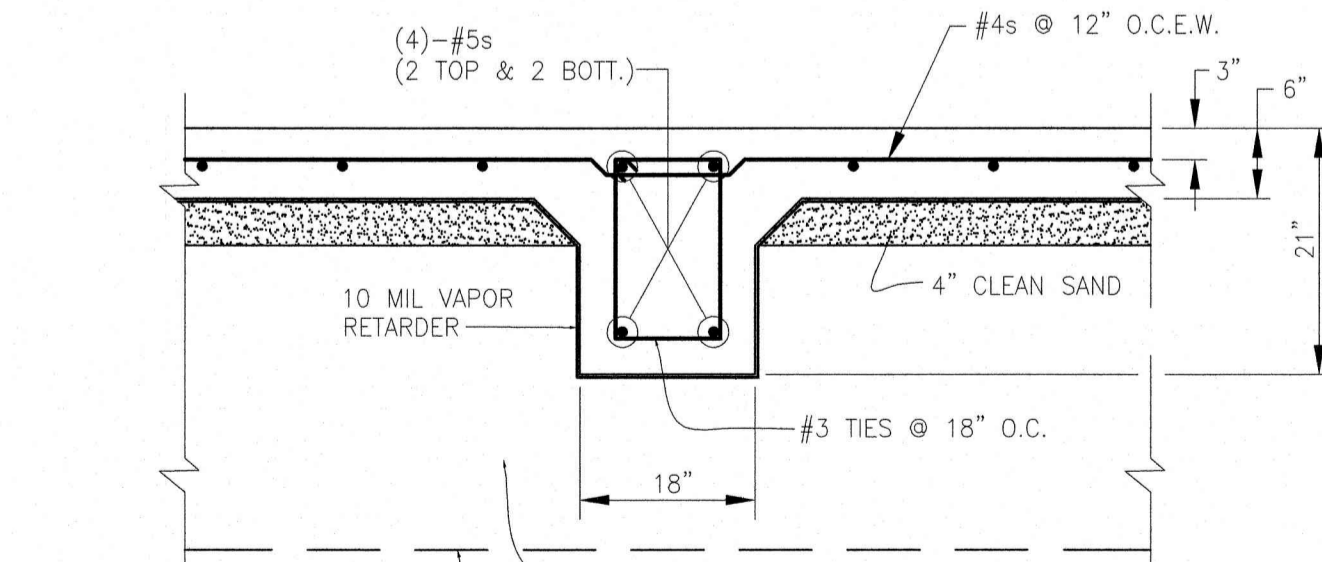
CAST-IN-PLACE CONCRETE

1. CAST-IN-PLACE CONCRETE SHALL BE NORMAL WEIGHT WITH A MINIMUM COMPRESSIVE STRENGTH OF 3,600 PSI AT 28 DAYS.
2. REINFORCED CONCRETE SHALL CONFORM TO ACI 318, LATEST EDITION.
3. REINFORCING BARS SHALL BE ASTM A 615 GRADE 60, WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185 AND SHALL BE SUPPLIED IN FLAT SHEETS. LAP FABRIC 2 MESHES AS SPLICES.
4. PROVIDE CORNER BARS IN CONCRETE GRADE BEAMS AT ALL CORNERS & INTERSECTIONS.
5. REQUIRED CONCRETE COVER FOR REINFORCING BARS:
DEPOSITED AGAINST EARTH: 3"
EXPOSED TO WEATHER OR EARTH AFTER FORMS REMOVED: 2"
6. CONTRACTOR SHALL VERIFY THE PRESENCE, LOCATION, SIZES, AND CORRECTNESS OF ALL OPENINGS, DEPRESSIONS, AND EMBEDMENTS PRIOR TO PLACING CONCRETE. NO OPENINGS SHALL BE PERMITTED THROUGH BEAMS UNLESS DETAILED OR APPROVED BY THE ENGINEER.
7. AS SOON AS FORMS ARE REMOVED, UNDESIRABLE FINIS AND OTHER PROJECTIONS SHALL BE REMOVED, OFFSETS LEVELED, AND VOID OR DAMAGED PLACES IMMEDIATELY SATURATED WITH WATER AND PREPARED WITH MORTAR OF THE SAME COMPOSITION AS USED IN THE MIX. EXPOSED SURFACES SHALL BE RUBBED WITH CARBORUNDUM STONE TO A SMOOTH FINISH FREE FROM FORM MARKS OR HONEYCOMBS IMMEDIATELY AFTER FORMS ARE REMOVED.
8. PLACE CONCRETE CONTINUOUSLY, WITHOUT INTERRUPTION, IN SUCH A MANNER AS NOT TO CAUSE SEGREGATION, VIBRATING EACH LIFT. USE CHUTES OR TREMIE FOR PLACING CONCRETE WHERE A DROP OF MORE THAN 5' IS REQUIRED, OR PUMP CONCRETE INTO PLACE.



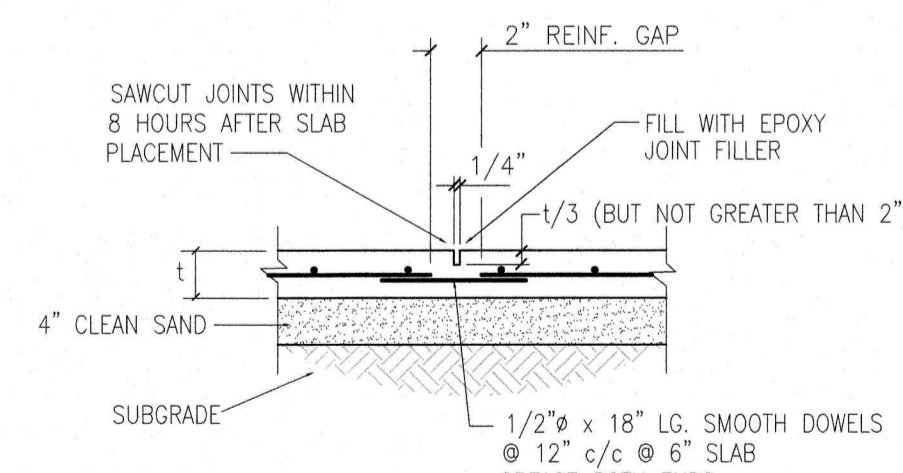
SECTION A-A

N.T.S.



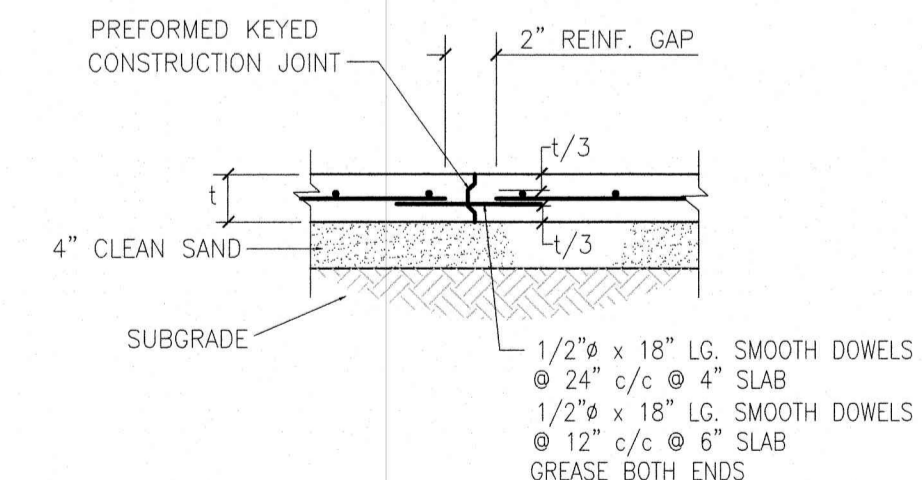
SECTION B-B

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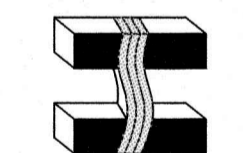
SAWED CONTROL JOINT DETAIL

N.T.S.



KEYED CONSTRUCTION JOINT DETAIL

N.T.S.



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